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- Generalities of Digital Twins
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  - Material Parameters
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- Thermal Effects
- Transient Cases
- Uncertainty

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Optimization

## WHAT IS A DIGITAL TWIN ?

#### AIAA Definition:

A Digital Twin is a set of virtual information constructs that mimics the structure, context, and behavior of an individual/unique physical asset, is dynamically updated with data from its physical twin throughout its lifecycle, and informs decisions that realize value.

American Institute of Aeronautics and Astronautics (AIAA), Digital Engineering Integration Committee. Digital Twin: Definition and Value. AIAA and AIA Position Paper, 2020. https://www.aiaa.org/docs/default-source/ uploadedfiles/issues-and-advocacy/ policy-papers/ digitaltwin-institute-position-paper-(december-2020).pdf.

#### WHAT IS A DIGITAL TWIN ?

- Real 'Thing': Object, Process, Patient, ...
- Digital Copy/Mirror
- Sensors
- Infer State of 'Thing' from Digital Copy + Sensors
- Update Digital Copy Throughout Lifecycle of 'Thing'

Digital Twins: Generalities 3

# EXAMPLE: BRIDGE (1)



3

# EXAMPLE: BRIDGE (2)

# Standard Forces, ...





Digital Twins: Generalities 5

# EXAMPLE: BRIDGE (3)



# Displacements, Velocities, Accelerations



# EXAMPLE: BRIDGE (4)

# Sensor Data + Model



Infer State of System



## DEFINITIONS: DIGITAL MODEL

# **Digital Model**



## DEFINITIONS: DIGITAL SHADOW

# **Digital Shadow**



## DEFINITIONS: DIGITAL TWIN

# **Digital Twin**



#### **DIGITAL TWINS:** Product of Megatrends

- Pervasive Use of CAD/Numerical Modeling
  - For Every Product/Process/Building/Patient/...  $\rightarrow$  Have **Detailed Data** of 'Real Thing'
- Pre-Compute, Only Then Build/Operate/Treat/...
  - Huge Reduction in Prototyping/Production Costs
  - Optimal Treatment of Patients
    - $\rightarrow$  Have **Detailed Model(s)** of 'Real Thing'
- Sensors Everywhere
  - Precise, Connected [G4,G5,...], Rugged, Cheap, ...
    - $\rightarrow$  Can **Measure** the 'Real Thing'

#### SENSORS FOR MONITORING BRIDGES



Courtesy: F. Schill, HS Mainz

#### FUTURE SENSORS FOR MONITORING CONCRETE



#### DIGITAL TWINS: Modus Operandi

- Have Digital Copy of Object/Process/Patient/...
- Equip Object/Process/Patient/... With Sensors
- With Data from Sensors (+Models): Infer State Normal, Weakening, Damaged, ...
- Update Digital Copy (+Models) Throughout Lifetime

#### DIGITAL TWINS: 'GREAT EXPECTATIONS'

- Increased Safety
- Increased Comfort
- Longer Life Cycles (Assets, Processes, Humans)
- Optimal Process Control
- Reduced Environmental Footprint

- ...

## DIGITAL TWINS: DATA LEVELS

- Description
- Enumeration of Parts
- Production/Replacement/Maintenance History
- Geometry for Display/CAD/Production
- Geometry/Abstraction for Modeling
  - Needs Proper Data for Each Discipline [CSD, CFD, CEM, CTD, ...]
  - Needs Proper Mesh/PDE Solver/... for Each Discipline
  - May Involve Extensive 'De-Featuring
- Computational Mechanics Data
  - Material Data, Mesh, BC, Loads, ROMs,

- ...

#### DTs: WHAT LEVEL OF ABSTRACTION ?

- DT Is Not Reality, Only Model of Reality
  - Not Computing Each Atom All The Time  $\Rightarrow$
- Need Abstraction Levels
- Partial/Ordinary Differential Equation(s)
  - CFD: Lifting Line/Potential/Euler/RANS/ LES/DNS
  - CSD: Lumped/Beam/Plate/Shell/Solid/
  - ...
- Numerics
  - FEM/FDM/FVM/...
- Model Abstraction from Numerics: ROMs/Surrogate Models
  - Modal, POD, PGD,...

#### DTs: CONSEQUENCES OF ABSTRACTION

- Level of Abstraction Determines:
  - Digital Twin
  - Data Needed for DT  $[\text{CAD} \rightarrow \text{DT}]$
  - Specialized Personnel Needed to Build DT
  - Software That Allows Seamless Updates
- Sensors
  - Type
  - Frequency of Measurements
  - Edge Computing

## $\rightarrow$ Determines Possible Type of Monitoring

#### DTs: DELUGE OF DATA

- DTs for Every Object/Process/Patient/
- Constant Sensor Data
- Constant Update of DTs
- $\rightarrow$  Deluge of Data
- Who ?
  - Stores
  - Secures/Insures
  - Manages
  - Retrieves/Compares/Updates
  - Curates
  - ...

#### DTs OF ALL OF US

- Every Click, Every Web Search, Every Call
- Build Digital Twin of Human Behaviour/Thoughts
- Exploit Economically to the Maximum Extent
  - Directly: Advertising
  - Indirectly: 'Time on Subject'
- Examples:
  - Online Merchants: Amazon, Walmart, ...
  - Web Search Engines
  - Social Media: Facebook, Instagram, TikTok, ...
  - Hardware/Software: Apple, Microsoft, ...

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  - Hardware/Software: Apple, Microsoft, ...
- 'We Know More About You Than You
- 'Surveillance Capitalism [1789, 1989, ...]

#### LINEAR ELASTICITY

Define:

- $\rho$ : Density
- **u**: Displacement

$$\mathbf{u} = (u_x, u_y, u_z)$$

-  $\sigma$ : Stress Tensor

$$\sigma = \begin{bmatrix} \sigma^{xx} & \sigma^{xy} & \sigma^{xz} \\ \sigma^{yx} & \sigma^{yy} & \sigma^{yz} \\ \sigma^{zx} & \sigma^{zy} & \sigma^{zz} \end{bmatrix}$$

-  $\epsilon$ : Strain Tensor

$$\epsilon = \frac{1}{2} \left[ \nabla \mathbf{u} + t \nabla \mathbf{u} \right]$$

$$\epsilon = \frac{1}{2} \begin{bmatrix} u_{x,x} + u_{x,x} & u_{x,y} + u_{y,x} & u_{x,z} + u_{z,x} \\ u_{y,x} + u_{x,y} & u_{y,y} + u_{y,y} & u_{y,z} + u_{z,y} \\ u_{z,x} + u_{x,z} & u_{z,y} + u_{y,z} & u_{z,z} + u_{z,z} \end{bmatrix}$$

# CONSERVATION OF MOMENTUM (1)



#### CONSERVATION OF MOMENTUM (2)

#### Forces: x-Direction

- x direction: x-Stress x Area at x:  $dy \ dz \ \sigma^{xx}$
- x direction: x-Stress x Area at x + dx:  $dy \ dz \ [\sigma^{xx} + (\sigma^{xx})_{,x} dx + O(dx^2)]$
- y direction: x-Shear x Area at y:  $dx \ dz \ \sigma^{yx}$
- y direction: x-Shear x Area at y + dy:  $dx \ dz \ [\sigma^{yx} + (\sigma^{yx})_{,y} dy + O(dy^2)]$
- z direction: x-Shear x Area at z:  $dx \ dy \ \sigma^{zx}$
- z direction: x-Shear x Area at z + dz:  $dx dy [\sigma^{zx} + (\sigma^{zx})_{,z}dz + O(dz^2)]$

External Forces:  $f_x$ 

Momentum Increment (Acceleration):

-  $dx dy dz \rho \frac{d^2 u_x}{dt^2}$ 

## CONSERVATION OF MOMENTUM (3)

Balance (x): Momentum Increment = Sum of Forces  $\Rightarrow$ 

$$dx \ dy \ dz\rho \ \frac{d^2 u_x}{dt^2} = dy \ dz(\sigma^{xx})_{,x}dx + dx \ dz(\sigma^{yx})_{,y}dy$$
$$+ dx \ dy(\sigma^{zx})_{,z}dz + f_x + O(dx^4)$$

 $\Rightarrow$  for  $dx, dy, dy \rightarrow 0$ :

$$\rho \ \frac{d^2 u_x}{dt^2} = \sigma_{,x}^{xx} + \sigma_{,y}^{yx} + \sigma_{,z}^{zx} + f_x$$

Same for y, z

 $\Rightarrow$  Newton's Law

$$\rho \ddot{\mathbf{u}} = \nabla \cdot \boldsymbol{\sigma} + \mathbf{f} \tag{E.1}$$

#### STRESS-STRAIN RELATIONSHIP

In General

$$\sigma = \mathbf{C}\epsilon$$

For Isotropic Material

$$\sigma = 2\mu\epsilon + \lambda tr(\epsilon)\mathbf{I}$$

$$\sigma^{ij} = \mu \left(\frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i}\right) + \lambda \frac{\partial u_k}{\partial x_k} \delta_{ij}$$

-  $\mu, \lambda$ : Lamé Parameters

Usual Material Parameters:

- Young's Modulus:

$$E = \frac{\mu(3\lambda + 2\mu)}{\lambda + \mu}$$

- Poisson's Ratio:

$$\nu = \frac{\lambda}{2(\lambda + \mu)}$$

#### STRESS-STRAIN RELATIONSHIP

Re-Write Constitutive Eqn. For Strains

$$\epsilon = \frac{1}{2\mu} \left[ \sigma - \frac{\lambda}{2\mu + 3\lambda} tr(\sigma) \mathbf{I} \right]$$
$$\epsilon_{ij} = \frac{1}{2\mu} \left[ \sigma^{ij} - \frac{\lambda}{2\mu + 3\lambda} \delta_{ij} \sigma^{kk} \right]$$

Special Case: Uniaxial Stress

-  $\sigma^{11} = T$ , All Other  $\sigma^{ij} = 0$ 

 $\Rightarrow$ 

$$\epsilon_{11} = \frac{1}{2\mu} \left[ \sigma^{11} - \frac{\lambda}{2\mu + 3\lambda} \sigma^{11} \right]$$

 $\Rightarrow$  (Hooke's Law)

$$\sigma^{11} = \frac{\mu(3\lambda + 2\mu)}{\lambda + \mu}\epsilon_{11}$$

# APPROXIMATION THEORY (1)

<u>To do</u>: given u(x) in  $\Omega$ , approximate by known functions

$$u(x) \approx u^h(x) = f^i(x)a_i = N^i(x)\hat{u}_i$$



Approximation of Functions

#### APPROXIMATION THEORY (2)

#### Examples:

- Truncated Taylor Series

$$u(x) \approx u^{h}(x) = a_0 + a_1 x + a_2 x^2 + \dots + a_m x^m$$

$$a_j = \frac{1}{j!} \left. \frac{d^j u}{dx^j} \right|_{x=0}$$

- Truncated Sine Series

$$u(x) \approx u^{h}(x) = a_{j} \sin \frac{j\pi x}{L}$$
$$a_{j} = \frac{2}{L} \int_{0}^{L} u(x) \sin \frac{j\pi x}{L} dx$$

- Legendre Polynomials
- Hermite Polynomials

In General: Choose complete set of trial functions  $N^j$ :

$$u(x) \approx u^h(x) = N^j a_j \quad ; \quad j = 1, 2, \dots M$$

#### DETERMINATION OF CONSTANTS

<u>1. Point Fitting</u>: set  $u^h = u$  at M selected points

 $\Rightarrow$ 

$$u^{h}|_{x_{k}} = u|_{x_{k}} \quad k = 1, 2, \dots M$$



#### 2. Weighted Residual Methods(WRM):

Define:  $\epsilon^h = u - u^h$  - the error or residual

Require:  $\epsilon^h \to 0$  in  $\Omega$ 

Introduce a set of weighting functions  $W^i$ ; i = 1, 2, ...MRequire that:

$$\int_{\Omega} W^i \epsilon^h d\Omega = 0 \quad , \ \ i=1,2,...M$$

Then, as  $M \to \infty$ ,  $\epsilon^h \to 0$  at all points in  $\Omega$ Insert expression for  $u^h$ :

$$\int_{\Omega} W^i \left( u - N^j a_j \right) d\Omega = 0$$

 $\Rightarrow$ 

$$K^{ij} = \int_{\Omega} W^i N^j d\Omega \quad , \quad r^i = \int_{\Omega} W^i u d\Omega$$

Choice of  $W^i$  defines method !

#### 2.1 Point Collocation

Choose:  $W^i = \delta(x - x_i)$ ,  $x_i \in \Omega$ 

WR statement

 $\Rightarrow$ 

$$\int_{\Omega} \delta(x - x_i) \epsilon^h d\Omega = \epsilon^h(x_i) = 0 \quad , \quad i = 1, 2, \dots M$$

$$N^j(x_i)a_j = u(x_i)$$

 $\Rightarrow$  same as Point Fitting

2.2 Galerkin Method

Choose:  $W^i = N^i$ 

WR statement

$$\int_{\Omega} N^{i} \epsilon^{h} d\Omega = \int_{\Omega} N^{i} \left( u - N^{j} a_{j} \right) d\Omega = 0 \quad , \quad i = 1, 2, \dots M$$

$$\left[\int_{\Omega} N^{i} N^{j} d\Omega\right] a_{j} = \int_{\Omega} N^{i} u d\Omega \quad , \quad i = 1, 2, \dots M$$

i.e. <u>Remarks</u>:

## LEAST SQUARES PROBLEM

$$I_{ls} = \int_{\Omega} (\epsilon^{h})^{2} d\Omega = \int_{\Omega} (u^{h} - u)^{2} d\Omega$$
$$= \int_{\Omega} (N^{k} a_{k} - u)^{2} d\Omega \to min$$

$$\delta I_{ls} = \delta a_k \int_{\Omega} N^k (N^l a_l - u) d\Omega = 0$$
$$\int_{\Omega} N^k N^l d\Omega \ a_l = \int_{\Omega} N^k u d\Omega$$

 $\Rightarrow$  Equivalent to Galerkin WRM

 $\Rightarrow$ 

 $\Rightarrow$  Choice of  $W^i$  from same set as  $N^i$  optimal

#### DRAWBACKS OF GLOBAL TRIAL FUNCTIONS

1. Determining  $N^j$ 's difficult for all but the simplest geometries in 2/3D

problems (can use strongly orthogonal polynomials)

4.  $a_j$ 's have no physical significance
## LOCAL TRIAL FUNCTIONS

Given u(x),  $x \in \Omega$ :

Divide  $\Omega$  into a set of non-overlapping sub-intervals  $\Omega_{el}$ 

Define  $u^h$  in each sub-interval

Sub-intervals - ELEMENTS

 $x_1, x_2, ...$  - NODES



## CONSTANT TRIAL FUNCTIONS

Define a piecewise constant function

$$P^E = \begin{cases} 1 & \text{in element } E\\ 0 & \text{in all other elements} \end{cases}$$



**Constant Trial Functions** 

Then globally

$$u \approx u^h = P^E u_E$$

Locally, on element el,

$$u \approx u^h = u_{el}$$

#### LINEAR TRIAL FUNCTIONS

Better approximation: let  $u^h$  vary linearly Place nodes at end of each element Define a piecewise linear trial function

$$N^{j} = \begin{cases} 1 & \text{at node } j \\ 0 & \text{at all other nodes} \end{cases}$$

and  $N^j$  non-zero only on elements associated with node j



Linear Trial Functions

Globally

$$u \approx u^h = N^j(x)u(x_j) = N^j(x)\hat{u}_j$$

Locally over element el with nodes 1 and 2

$$u \approx u^h = N^1 \hat{u}_1 + N^2 \hat{u}_2$$

$$\xi = (x - x_1)/(x_2 - x_1)$$

$$N_{el}^1 = (x_2 - x)/h_{el} = 1 - \xi$$

$$N_{el}^2 = (x - x_1)/h_{el} = \xi$$

$$u^h = \frac{(x_2 - x)u_1 + (x - x_1)u_2}{h_{el}} = (1 - \xi)u_1 + \xi u_2$$

Observe that:  $x = (1 - \xi)x_1 + \xi x_2 = N^1 x_1 + N^2 x_2$ 

## QUADRATIC TRIAL FUNCTIONS

Better approximation: let  $u^h$  vary quadratically Place nodes at end of each element, as well as the middle



Quadratic Trial Functions

$$N_{el}^{1} = (1 - \xi)(1 - 2\xi)$$
$$N_{el}^{2} = 4\xi(1 - \xi)$$
$$N_{el}^{3} = -\xi(1 - 2\xi)$$

#### GENERAL PROPERTIES OF SHAPE-FUNCTIONS

1. <u>Interpolation Property</u>:

 $\Rightarrow$ 

$$u^h = N^i(x)\hat{u}_i$$

 $u^h(x_j) = N^i(x_j)\hat{u}_i = \hat{u}_j \Rightarrow N^i(x_j) = \delta^i_j$ 

2. <u>Constant Sum</u>: Must be able to represent a constant

 $u = 1 \Rightarrow u^h = 1 = N^i(x)\hat{u}_i$ 

but interpolation property  $\Rightarrow \hat{u}_i = 1 \Rightarrow$ 

$$\sum_{i} N^{i}(x) = 1 \quad , \quad \forall x \in \Omega \tag{(*)}$$

3. <u>Conservation property</u>: from Eqn.(\*):

$$\sum_{i} N^{i}_{,k} = 0 \quad , \quad \forall x \in \Omega_{el}$$

# WRM OF APPROXIMATION WITH LOCAL FUNCTIONS <u>Basic Idea</u>:

$$\int_{\Omega} \dots = \sum_{el} \int_{\Omega_{el}} \dots \tag{*}$$

 $\Rightarrow$ 

- build integrals on element level
- gather info from global point-arrays to local elementarrays
- scatter-add resulting integrands to global rhs/matrix locations

$$K^{ij}u_j = \left[\sum_{el} K^{ij}_{el}\right] \left[u_j\right]_{el} = \sum_{el} r^i_{el} = r^i$$

<u>Note</u>: for Eqn.(\*) only need info: nodes belonging to an element

 $\Rightarrow$  drastic simplification of data structures/logic

#### FROM APPROXIMATION TO OPERATORS

<u>Before</u>: given u, approximate:  $||u - u^h|| \to min$ 

<u>Now:</u> given L(u) = 0 approximate:  $||L(u) - L(u^h)|| \to min$ 

$$\Rightarrow \qquad \qquad \|L(u^h)\| \to \min$$

Minimize error:

$$\epsilon_L^h = L(u^h) = L(N^i \hat{u}_i)$$

using WRM

$$\int_{\Omega} W^{i} \epsilon_{L}^{h} d\Omega = 0 \quad , \quad i = 1, M$$

Choice of  $N^i, W^i$  defines the method:

- $N^i$  polynomial,  $W^i = \delta(x_i)$  : FDM
- $N^i$  polynomial,  $W^i = 1$  if  $\mathbf{x} \subset \Omega_{el}$ , 0 otherwise : FVM
- $N^i$  polynomial,  $W^i = N^i$  : GFEM
- $N^i$  polynomial,  $W^i \neq N^i$  : Petrov-GFEM
- $N^i$  spectral,  $W^i = \delta(x_i)$  : SEM
- $N^i, W^i$  NURBS: Isogeometric G/PG

#### POISSON OPERATOR

Given

$$abla^2 u = f \quad in \quad \Omega \quad , \quad u = 0 \quad on \quad \Gamma$$

#### WRM

$$\int_{\Omega} W^i \, \nabla^2 N^j \, d\Omega \, \hat{u}_j = \int_{\Omega} W^i \, f \, d\Omega$$

 $\Rightarrow$ 

- $N^j$  should have defined 2nd order derivatives  $\Rightarrow C^1$ -continuous across elements
- $W^i$  can be the  $\delta$ -function

#### Integration by parts:

$$-\int_{\Omega} \nabla W^{i} \cdot \nabla N^{j} \ d\Omega \ \hat{u}_{j} = \int_{\Omega} W^{i} \ f \ d\Omega$$

 $\Rightarrow$ 

- Order of max(derivative) reduced  $\Rightarrow$  can use wider space of trial/weight functions
- $N^j$  should have defined 1st order derivatives  $\Rightarrow C^0$ -continuous across elements
- $W^i$  can not be the  $\delta$ -function

#### MINIMIZATION PROBLEM

$$I_{rz} = \int_{\Omega} [\nabla \epsilon^h]^2 \ d\Omega = \int_{\Omega} [\nabla (u^h - u)]^2 \ d\Omega \to min$$

$$\delta I_{rz} = \delta \hat{u}_i \int_{\Omega} \nabla N^i \cdot (\nabla N^j \hat{u}_j - \nabla u) \ d\Omega = 0$$

but

 $\Rightarrow$ 

$$-\int_{\Omega} \nabla N^{i} \cdot \nabla u \ d\Omega = \int_{\Omega} N^{i} \nabla^{2} u \ d\Omega = \int_{\Omega} N^{i} f \ d\Omega$$

$$\Rightarrow$$

$$\delta I_{rz} = \delta \hat{u}_i \left[ \int_{\Omega} \nabla N^i \cdot \nabla N^j \ d\Omega \ \hat{u}_j + \int_{\Omega} N^i \ f \ d\Omega \right] = 0$$

- $\Rightarrow$  equivalent to Galerkin WRM
- $\Rightarrow$  choice of  $W^i$  from same set as  $N^i$  optimal

# 1D ELASTICITY (1)

$$-\frac{\partial}{\partial x}E\frac{\partial u}{\partial x} = q$$

Approximate:  $u = N^i \hat{u}_i$ 

WRM+Integration by parts+Galerkin:

$$\int_{\Omega} \frac{\partial N^i}{\partial x} E \frac{\partial N^j}{\partial x} \, dx \, \hat{u}_j = \int_{\Omega} N^i N^j \, dx \, \hat{q}_j$$

# 1D ELASTICITY (2)

For Linear Shape Functions:

$$\mathbf{K}_{el} = \int_{\Omega} \frac{\partial N^{i}}{\partial x} E \frac{\partial N^{j}}{\partial x} dx = \frac{E}{\Delta x} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$
$$\mathbf{M}_{el} = \int_{\Omega} N^{i} N^{j} dx = \frac{\Delta x}{6} \begin{bmatrix} 2 & 1 \\ 1 & 2 \end{bmatrix}$$
$$\mathbf{M}_{el}^{l} = \frac{\Delta x}{2} \begin{bmatrix} 1 & 0 \\ 0 & 1 \end{bmatrix}$$

#### REFERENCES

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## MOTIVATION (1)

- All Structures Age
- 1st World: Concrete [Bridges, Buildings, ...]
  - Life Span: 60-80 Years (Weathering, Cracks, ...)
  - Many Bridges and Buildings Nearing That Age

## $\rightarrow$ Infrastructure Crisis

- Same for Many Other Applications
  - Turbines
  - Composites [Wings, Propellers, ...]
  - Objects Subjected to Harsh Environments

## RECENT BRIDGE FAILURES



## RECENT BRIDGE FAILURES



#### MOTIVATION (2)

- As All Structures Age:
- Q1: Given Loads and Measurements: Can We Infer State of Material (Weakening) ?
- Q2: If We Know/Suspect Weakening: Where Should We Reinforce ?

## MOTIVATION (3)

- Problem of High Importance
  - $\Rightarrow$  Considerable Body of Work
- Frequency Domain (Sensors: Accelerometers)
  - Since Mid-70s
  - Large Effort at Sandia National Labs
  - Easy to Detect **That**, But Not **Where**
- Time Domain
  - Several (Some Adjoints in Time-Domain)
- Steady: Adjust/Approximate FEM Model from Measurements
  - Ladeveze et al.
  - Aubry et al. (Adjoints+Patches)
- New Here: DT, Continuous Monitoring, High-Fidelity

#### ASSUMPTIONS (1)

- Monitoring Via Loads and Measurements:
  - n (Standard) Loads  $\mathbf{f}^i, i = 1, n$  Given
  - n Displacements/Strains Measured at m Locations
- Weakening Can Occur at Any Location
  - Most Conservative
- Weakening Described by Field  $0 < \alpha(\mathbf{x}) < 1$
- Deformations and Strains Well Described by FEM:

$$\mathbf{K} \cdot \mathbf{u}_i = \mathbf{f}_i \quad , \quad i = 1, n \quad , \quad \mathbf{K} = \sum_{el} \alpha_{el} \mathbf{K}_{el}$$

## ASSUMPTIONS (2)

- Sensors Limited by Signal/Noise Ratios

 $|\mathbf{u}^m| > u_0 \quad , \quad |\mathbf{s}^m| > s_0$ 

- Forces Used to Monitor Structure Limited by Practical Considerations
  - $\Rightarrow$  Not Arbitrary

## LIMITATIONS (1)

Simple Truss, Clamped at x = 0, Force  $f_c$  Applied at x = LElasticity Equations Reduce to:

$$f = f_c$$
,  $E\frac{du}{dx} = f_c$ 

Integrate:

$$\frac{du}{dx} = \frac{f_c}{E}$$

 $\Rightarrow$ 

$$u(L) - u(0) = f_c \int_0^L \frac{1}{E} dx$$

Or:

$$\int_0^L \frac{1}{E} dx = \frac{u(L) - u(0)}{f_c}$$

#### LIMITATIONS (2)

Suppose:

- $f_c$ : Known (Measured)
- u(L): Known (Measured)

Q: Can We Determine E(x) ?

A: Only If  $E(x) = E_c$  (i.e. Constant) !

A: Not If E(x) Arbitrary

Consider *n* Regions  $\Delta x_i, i = 1, n$ ;  $E_i$  Constant in Each Region

 $\Rightarrow$ 

$$\sum_{i} \frac{\Delta x_i}{E_i} = \frac{u(L) - u(0)}{f_c}$$

 $\Rightarrow$  Infinitely Many Possible Solutions

#### LIMITATIONS (3)

Simple Case:

- $L = 1, u(0) = 0, u(1) = 1, f_c = 1$
- 4 Equal Regions of  $\Delta x = 0.25$  $\Rightarrow$

$$\sum_{i=1}^4 \frac{1}{k_i} = 4$$

Options:

a) 
$$E_1 = E_2 = E_3 = E_4 = 1;$$
  
b)  $E_1 = E_2 = 2; E_3 = E_4 = 2/3;$   
c)  $E_1 = 10, E_2 = 5, E_3 = 2, E_4 = 10/32;$   
d) ...

Moreover: Spatial Sequence of  $E^\prime s$  Can be Changed Without Affecting the Resulting u(L)

 $\Rightarrow$  Highly Oscillatory Distributions of E Admissible





1D Case: Possible E-Modules and Resulting Deformations for  $f_c = 1$ 





1D Case: Possible E-Modules and Resulting Deformations for  $f_c = 1$ 

#### FORMULATE AS OPTIMIZATION PROBLEM

Find  $\alpha(\mathbf{x})$  Such That

$$I(\mathbf{u}_{1,\dots,n},\alpha) = \frac{1}{2} \sum_{i=1}^{n} \sum_{j=1}^{m} w_{ij}^{md} (\mathbf{u}_{ij}^{md} - \mathbf{I}_{ij}^{d} \cdot \mathbf{u}_{i})^{2}$$
$$+ \frac{1}{2} \sum_{i=1}^{n} \sum_{j=1}^{m} w_{ij}^{ms} (\mathbf{s}_{ij}^{ms} - \mathbf{I}_{ij}^{s} \mathbf{s}_{i})^{2} \to min$$

Subject To:

$$\mathbf{K} \cdot \mathbf{u}_i = \mathbf{f}_i \quad , \quad i = 1, n \quad , \quad \mathbf{K} = \sum_{el} \alpha_{el} \mathbf{K}_{el}$$

#### EXTEND TO LAGRANGIAN FUNCTIONAL

$$L(\mathbf{u}_{1,\dots,n},\alpha,\tilde{\mathbf{u}}_{1,\dots,n}) = I(\mathbf{u}_{1,\dots,n},\alpha) + \sum_{i=1}^{n} \tilde{\mathbf{u}}_{i}^{t}(\mathbf{K}\cdot\mathbf{u}_{i} - \mathbf{f}_{i})$$

-  $\tilde{\mathbf{u}}:$  Adjoint Variables

Variation of the Lagrangian WRT Each Component

$$\begin{aligned} \frac{dL}{d\tilde{\mathbf{u}}_i} &= \mathbf{K} \cdot \mathbf{u}_i - \mathbf{f}_i = 0\\ \frac{dL}{d\mathbf{u}_i} &= \sum_{j=1}^m w_{ij}^{md} \mathbf{I}_{ij}^d \cdot (\mathbf{u}_{ij}^{md} - \mathbf{I}_{ij}^d \cdot \mathbf{u}_i)\\ &+ \sum_{j=1}^m w_{ij}^{ms} \mathbf{J}_{ij}^s (\mathbf{s}_{ij}^{ms} - \mathbf{I}_{ij}^s \mathbf{s}_i) + \mathbf{K}^t \tilde{\mathbf{u}}_i = 0\\ \frac{dL}{d\alpha_e} &= \sum_{i=1}^n \tilde{\mathbf{u}}_i^t \cdot \frac{d\mathbf{K}}{d\alpha_e} \cdot \mathbf{u}_i = \sum_{i=1}^n \tilde{\mathbf{u}}_i^t \cdot \mathbf{K}^e \cdot \mathbf{u}_i \quad, \end{aligned}$$

-  $\mathbf{J}_{ij}^s$ : Relationship Displacements/Strains

## USE OF ADJOINT SOLVERS

- Consequences
  - Gradient of L wrt  $\alpha$ : n Forward/Adjoint Solves;
  - **Independent** of Number of Variables of  $\alpha$  (!);
  - Once Forward/Adjoint Solves Done: Cost of Gradient of Each  $\alpha_{el}$  Is O(1);

 $\Rightarrow$  Can Use Detailed FEM Models  $\rightarrow$  Detailed Digital Twin

- 'Hi-Fi Digital Twin'
- Based on Algebraic Equations (FEMs), Not PDEs  $\Rightarrow$  General
- Most Structural Problems:  $\mathbf{K}=^t\mathbf{K}\Rightarrow$ 
  - Direct Solvers: Cost of Adjoint Negligible
  - Iterative Solver: Preconditioner Can Be Re-Utilized

#### OPTIMIZATION CYCLE

- For Each Force/Measurement Pair i:
- With Current  $\alpha$ : Obtain Displacements  $\rightarrow \mathbf{u}_i$
- With Current  $\alpha$ ,  $\mathbf{u}_i$ ,  $\mathbf{u}_{ij}^{md}$ ,  $\mathbf{s}_{ij}^{md}$ : Obtain Adjoints  $\rightarrow \tilde{\mathbf{u}}_i$
- With  $\mathbf{u}_i, \tilde{\mathbf{u}}_i$ : Obtain Gradients  $\rightarrow I^i_{,\alpha} = L^i_{,\alpha}$
- Sum up the gradients  $\rightarrow I_{,\alpha} = \sum_{i=1}^{n} I_{,\alpha}^{i}$
- If Necessary: Smooth Gradients  $\rightarrow I^{smoo}_{,\alpha}$

- Update 
$$\alpha_{new} = \alpha_{old} - \gamma I^{smoo}_{,\alpha}$$
.

-  $\gamma$ : Stepsize

#### INTERPOLATION OF DISPLACEMENTS AND STRAINS

Displacements:

$$\mathbf{u}_i(\mathbf{x}_i^m) = \mathbf{I}_i^d(\mathbf{x}_i^m) \cdot \mathbf{u}$$

Strains:

$$\mathbf{s} = \mathbf{D}\mathbf{u}$$

$$\mathbf{s}_i(\mathbf{x}_i^m) = \mathbf{I}_i^s(\mathbf{x}_i^m) \cdot \mathbf{s} = \mathbf{I}_i^s(\mathbf{x}_i^m) \cdot \mathbf{D} \cdot \mathbf{u}$$

## CHOICE OF WEIGHTS (1)

Cost Function:

$$I(\mathbf{u}_n, \alpha) = \frac{1}{2} \sum_{i=1}^n \sum_{j=1}^m w_{ij}^{md} (\mathbf{u}_{ij}^{md} - \mathbf{I}_{ij}^d \cdot \mathbf{u}_i)^2$$
$$+ \frac{1}{2} \sum_{i=1}^n \sum_{j=1}^m w_{ij}^{ms} (\mathbf{s}_{ij}^{ms} - \mathbf{I}_{ij}^s \mathbf{s}_i)^2$$

Problem: Dimensionally Inconsistent (!)

#### CHOICE OF WEIGHTS (2)

Local Weighting:

$$w_{ij}^{md} = \frac{1}{(\mathbf{u}_{ij}^{md})^2} \quad ; \quad w_{ij}^{ms} = \frac{1}{(\mathbf{s}_{ij}^{ms})^2}$$

- Works Well

 May Lead to 'Over-Emphasis' of Small Displacements/Strains (That May Be In Regions of Marginal Interest)

#### CHOICE OF WEIGHTS (3)

Average Weighting

$$u_{av} = \frac{\sum_{j=1}^{m} |\mathbf{u}_{ij}^{md}|}{m} \quad ; \quad w_{ij}^{md} = \frac{1}{u_{av}^2}$$
$$s_{av} = \frac{\sum_{j=1}^{m} |\mathbf{s}_{ij}^{ms}|}{m} \quad ; \quad w_{ij}^{ms} = \frac{1}{s_{av}^2}$$

- Works Well
- May Lead to 'Under-Emphasis' of Small Displacements/Strains (That May Be In Important Regions)

#### CHOICE OF WEIGHTS (4)

Max Weighting

$$u_{max} = max(|\mathbf{u}_{ij}^{md}|, j = 1, m) \; ; \; w_{ij}^{md} = \frac{1}{u_{max}^2}$$
$$s_{max} = max(|\mathbf{s}_{ij}^{ms}|, j = 1, m) \; ; \; w_{ij}^{ms} = \frac{1}{s_{max}^2}$$

- Works Well
- May Lead to 'Under-Emphasis' of Smaller Displacements/Strains (That May Be In Important Regions)

#### CHOICE OF WEIGHTS (5)

Local/Max Weighting

$$w_{ij}^{md} = \frac{1}{max(\epsilon u_{max}, |\mathbf{u}_{ij}^{md}|))^2}$$
$$w_{ij}^{ms} = \frac{1}{max(\epsilon s_{max}, |\mathbf{s}_{ij}^{ms}|))^2}$$

- $\epsilon = O(0.01 0.10)$
- Combines Local Weighting With Max-Bound Minimum for Local Values
- Seemed to Work Best

## SMOOTHING OF GRADIENTS (1)

Gradient of  $\alpha$  Oscillatory, 'Lives in  $H^{-1}$ '

 $\Rightarrow$  Need to Smooth, 'Regularize'

Can Smooth  $\alpha$  or  $I_{,\alpha}$ 

Consider  $\alpha$
#### SMOOTHING OF GRADIENTS (2)

Option 1: Simple Element/Point/Element Averaging Step 1: Element to Point

$$\alpha_p = \frac{\sum_e \alpha_e V_e}{\sum_e V_e}$$

Step 2: Point to Element

$$\alpha_e = \frac{1}{n_e} \sum_i \alpha_i$$

- $\alpha_e$ : Weakness in Element e
- $V_e$ : Volume of Element e
- $\sum_{e}$ : All Elements Surrounding Point p
- $\alpha_p$ : Weakness Averaged to Point p
- $n_e$ : Nr. of Nodes/Points of Element e
- Simple, 'Crude'
- Works Surprisingly Well

#### SMOOTHING OF GRADIENTS (3)

Option 2:  $H^1$  (Weak) Laplacian Smoothing

$$\left[1 - \lambda \nabla^2\right] \alpha = \alpha_0 \quad , \quad \alpha_{,n} \big|^{\Gamma} = 0$$

- $\alpha_0$ : Initial, Unsmoothed Values
- $\alpha$ : Smoothed Values
- $\lambda$ : Free Parameter (Problem and Mesh Dependent

After Discretization:

$$\left[\mathbf{M}_{c}+\lambda\mathbf{K}_{d}
ight] oldsymbol{a}=\mathbf{M}_{p1p0}oldsymbol{a}_{0}$$

- $\mathbf{M}_c$ : Consistent Mass Matrix (L2 Matrix)
- $\mathbf{K}_d$ : 'Diffusion Matrix' (Laplacian)
- $\mathbf{M}_{p1p0}$ : Projection Matrix (Element  $\rightarrow$  Points)
- Solved Iteratively (Fast Convergence)
- Problem: Choice of  $\lambda$

#### SMOOTHING OF GRADIENTS (4)

**Option 3: Pseudo-Laplacian Smoothing** 

$$\left[1 - \lambda \nabla h^2 \nabla\right] \alpha = \alpha_0$$

- h: Characteristic Element Size

After Discretization:

$$\left[\mathbf{M}_{c} + \lambda \mathbf{K}_{h^{2}}\right] \boldsymbol{\alpha} = \mathbf{M}_{p1p0} \boldsymbol{\alpha}_{0}$$

-  $\mathbf{K}_{h^2}$ : ' $h^2$ -Scaled Diffusion Matrix' ( $h^2$  Laplacian) For Linear Elements (Simplex) Equivalent To

 $\left[\mathbf{M}_{c} + \lambda \left(\mathbf{M}_{l} - \mathbf{M}_{c}\right)\right] \boldsymbol{\alpha} = \mathbf{M}_{p1p0} \boldsymbol{\alpha}_{0}$ 

-  $\mathbf{M}_l$ : Lumped Mass Matrix - Typical Value:  $\lambda = 0.05$ 

#### SMOOTHING OF GRADIENTS (5)

Option 4: Smoothing via Convolution

$$\alpha = \frac{\int_{\Omega_h} G(\mathbf{x}) \alpha_0 d\Omega}{\int_{\Omega_h} G(\mathbf{x}) d\Omega}$$

- $\alpha_0$ : Initial, Unsmoothed Values
- $\alpha$ : Smoothed Values
- $\Omega_h$ : Local Domain
- $G(\mathbf{x})$ : Kernel Function (e.g. Gaussian)

Obervations:

- Choice of Local Domain
  - 3-4 Elements ? Constant ?
- Similar to Pseudo-Laplacian Smoothing

#### SMALL EXAMPLE: 1-D TRUSS (1)

Define Case:

- 1-D
- E, A Constant
- u(0) = 0
- One Load Only at x = L:  $f(L) = f_0$
- 2 Linear Elements
- Displacement Sensors at Each Point
- Weights for Displacements:  $w_i = 1.0$
- Assume: Structure Weakened to  $\alpha = 0.5$
- $\Rightarrow$  Exact Solution (Proper E, A, L):

 $u_0 = 0, u_1 = 1, u_2 = 2$ 

 $- \Rightarrow$  Measurements:

 $u_0^m = 0, u_1^m = 1, u_2^m = 2$ 

- Then: Exact Solution for  $\alpha = 1.0$ 

$$u_0 = 0, u_1 = 0.5, u_2 = 1$$

#### SMALL EXAMPLE: 1-D TRUSS (2)

First Iteration: Start With  $\alpha = 1.0$  (No Weakening)

$$\frac{dL}{d\tilde{\mathbf{u}}} = \mathbf{K} \cdot \mathbf{u} - \mathbf{f} = 0$$

 $\Rightarrow$ 

$$u_0^{FEM} = 0, u_1^{FEM} = 0.5, u_2^{FEM} = 1$$

Adjoint:

$$\frac{dL}{d\mathbf{u}} = \sum_{j=1}^{m} w_j^{md} \mathbf{I}_j^d \cdot (\mathbf{u}_j^{md} - \mathbf{I}_j^d \cdot \mathbf{u}) + \mathbf{K}^t \tilde{\mathbf{u}} = 0$$

As Measuring Points Coincident With FEM Points:

$$\mathbf{I}_j^d(x_i) = \delta_{ij}$$

#### SMALL EXAMPLE: 1-D TRUSS (3)

 $\Rightarrow \text{RHS for System } \mathbf{K}^{t} \tilde{\mathbf{u}} = \mathbf{r}$   $- r_{0} = u_{0}^{m} - u_{0}^{FEM} = 0.0$   $- r_{1} = u_{1}^{m} - u_{1}^{FEM} = 0.5$   $- r_{2} = u_{2}^{m} - u_{2}^{FEM} = 1.0$   $\Rightarrow \tilde{u}_{0} = 0, \tilde{u}_{1} > 0, \tilde{u}_{2} > \tilde{u}_{1} > 0$   $\frac{dL}{d\alpha_{e}} = \tilde{\mathbf{u}}^{t} \cdot \frac{d\mathbf{K}}{d\alpha_{e}} \cdot \mathbf{u} = \tilde{\mathbf{u}}^{t} \cdot \mathbf{K}^{e} \cdot \mathbf{u}$ 

Recall:  $\mathbf{K}^e$ 

$$\mathbf{K}_{el} = \frac{E}{\Delta x} \begin{bmatrix} 1 & -1\\ -1 & 1 \end{bmatrix}$$

 $\Rightarrow \frac{dL}{d\alpha_e} > 0$ 

Move in Direction Opposite to Gradient  $\Rightarrow$  Decrease  $\alpha$ 

### Crane

- Truss Elements
- Forces at Extreme Ends of Arm
- 10 Measurement Points
  - Displacements
  - Strains
- Smoothing: Simple Averaging
  - Element  $\rightarrow$  Point  $\rightarrow$  Element [No Volume Considerations]
  - Separate Smoothing of Gradients and Strength Factors
- FEELAST



### Crane: 1 Load, <u>Strains</u> Grad Smoothing



#### **Crane: 1 Load, <u>Strains</u>** Grad Smoothing, DOF: Tower Only



# Bridge

#### Dimensions: 40x5x10m





Material: Steel Trusses: A=1-100cm<sup>2</sup> FEELAST

#### Forces [+Gravity]

# Bridge



#### Displacements + Int Forces

Target StrFac

#### Bridge



### Footbridge



## Footbridge







### Footbridge





#### FEELAST

## Plate With Hole: Small Damaged Region









### **Plate With Hole**



### **Convergence Study**





































# **Plate With Hole 3D**

- 16Kels
- 120Kels
- X=0: Clamped
- X=Xmax: Fx
- FEELAST



## **Plate With Hole 3D**



## **Plate With Hole 3D**



### **Hoover Dam**



#### **Hoover Dam**



### **Hoover Dam: 51 Sensors**



#### FEELAST

#### **Hoover Dam: 51 Sensors**



Target

#### Detected/Recovered

### **Hoover Dam: 51 Sensors**



# **Concerto Bridge**

- Built in 2005 by iBMB TU Braunschweig for Testing Measurement Technologies
- Loads Can be Applied Using Hydraulic Presses and External Tendons
- Constructed with Prestressed, Post-Tensioned Concrete





# **Concerto Bridge**

#### **Configuration: KRATOS**

- Concrete: 77k Small Displacement Hex Elements
- Tendons: 800 Truss Elements
- 100 Potential Displacement (x) Sensors





### Blade

#### Siemens Project: xDT Titanium Blade Courtesy S. Vettori and E. Di Lorenzo



#### **SIEMENS**

### Blade

nelem=34K (Hex,Pr)
nsens=19
nload= 1
CALCULIX


# Blade

nelem=34K (Hex,Pr)
nsens=19
nload= 1
CALCULIX



# Is The Solution Stable ?

- Start From Close to the Exact Solution
- See If Exact Solution Is Obtained
- Here: Start With 0.2/0.5

# **Bridge: Start From 0.5**



# **Bridge: Start From 0.2**



# **Bridge: Start From 0.2**



#### EXTENSION TO TRANSIENT PROBLEMS

Find  $\alpha(\mathbf{x})$  Such That

$$I(\mathbf{u}_{1,..,n},\alpha) = \frac{1}{2} \int_0^T \sum_{i=1}^n \sum_{j=1}^m w_{ij}^{md} (\mathbf{u}_{ij}^{md} - \mathbf{I}_{ij}^d \cdot \mathbf{u}_i)^2 dt$$
$$+ \frac{1}{2} \int_0^T \sum_{i=1}^n \sum_{j=1}^m w_{ij}^{ms} (\mathbf{s}_{ij}^{ms} - \mathbf{I}_{ij}^s \mathbf{s}_i)^2 dt \to min$$

Subject To:

$$\begin{split} \mathbf{M} \cdot \ddot{\mathbf{u}}_i + \mathbf{C} \cdot \dot{\mathbf{u}}_i + \mathbf{K} \cdot \mathbf{u}_i &= \mathbf{f}_i \quad , \quad i = 1, n \quad , \\ \mathbf{u}_i(t=0) &= \mathbf{u}_i^0 \quad , \quad i = 1, n \quad , \\ \dot{\mathbf{u}}_i(t=0) &= \mathbf{v}_i^0 \quad , \quad i = 1, n \quad , \end{split}$$

And As Before:

$$\mathbf{K} = \sum_{el} \alpha_{el} \mathbf{K}_{el}$$

#### EXTEND TO LAGRANGIAN FUNCTIONAL

$$L(\mathbf{u}_{1,\dots,n},\alpha,\tilde{\mathbf{u}}_{1,\dots,n}) = I(\mathbf{u}_{1,\dots,n},\alpha)$$

$$+\int_0^T \sum_{i=1}^n \tilde{\mathbf{u}}_i^t (\mathbf{M} \cdot \ddot{\mathbf{u}}_i + \mathbf{C} \cdot \dot{\mathbf{u}}_i + \mathbf{K} \cdot \mathbf{u}_i - \mathbf{f}_i) dt$$

-  $\tilde{\mathbf{u}}:$  Adjoint Variables

+

Variation of the Lagrangian WRT Each Component

$$\begin{split} \frac{dL}{d\tilde{\mathbf{u}}_{i}} &= \mathbf{M} \cdot \ddot{\mathbf{u}}_{i} + \mathbf{C} \cdot \dot{\mathbf{u}}_{i} + \mathbf{K} \cdot \mathbf{u}_{i} - \mathbf{f}_{i} = 0 \\ \frac{dL}{d\mathbf{u}_{i}} &= \sum_{j=1}^{m} w_{ij}^{md} \mathbf{I}_{ij}^{d} \cdot (\mathbf{u}_{ij}^{md} - \mathbf{I}_{ij}^{d} \cdot \mathbf{u}_{i}) \\ \sum_{j=1}^{m} w_{ij}^{ms} \mathbf{J}_{ij}^{s} (\mathbf{s}_{ij}^{ms} - \mathbf{I}_{ij}^{s} \mathbf{s}_{i}) + \mathbf{M}^{t} \cdot \ddot{\ddot{\mathbf{u}}}_{i} - \mathbf{C}^{t} \cdot \dot{\ddot{\mathbf{u}}}_{i} + \mathbf{K}^{t} \cdot \ddot{\mathbf{u}}_{i} = 0 \\ \frac{dL}{d\alpha_{e}} &= \int_{0}^{T} \sum_{i=1}^{n} \tilde{\mathbf{u}}_{i}^{t} \cdot \frac{d\mathbf{K}}{d\alpha_{e}} \cdot \mathbf{u}_{i} dt = \int_{0}^{T} \sum_{i=1}^{n} \tilde{\mathbf{u}}_{i}^{t} \cdot \mathbf{K}^{e} \cdot \mathbf{u}_{i} dt \quad , \\ - \mathbf{J}_{ij}^{s}: \text{ Relationship Displacements/Strains} \end{split}$$

#### OPTIMIZATION CYCLE

- For Each Force/Measurement Pair i:
- With Current  $\alpha$ : Forward Solution, Obtaining Displacements  $\rightarrow \mathbf{u}_i(t)$
- With Current  $\alpha$ ,  $\mathbf{u}_i(t)$ ,  $\mathbf{u}_{ij}^{md}$ ,  $\mathbf{s}_{ij}^{md}$ : Backwards Solution, Obtaining Adjoints  $\rightarrow \tilde{\mathbf{u}}_i(t)$
- With  $\mathbf{u}_i(t), \tilde{\mathbf{u}}_i(t)$ : Obtain Gradients  $\rightarrow I^i_{,\alpha} = L^i_{,\alpha}$
- Sum up the gradients  $\rightarrow I_{,\alpha} = \sum_{i=1}^{n} I_{,\alpha}^{i}$
- If Necessary: Smooth Gradients  $\rightarrow I^{smoo}_{,\alpha}$
- Update  $\alpha_{new} = \alpha_{old} \gamma I_{,\alpha}^{smoo}$ .
- $\gamma$ : Stepsize

## EXAMPLE: BEAM (1)

- Beam
- 10 Elements
- Displacement Sensors at Each Point
- CALCULIX

(a) Undamaged. $t = 0.25$ seconds.	(b) Damaged. $t = 0.25$ seconds.
(c) Undamaged. $t = 0.50$ seconds.	(d) Damaged. $t = 0.50$ seconds.
(e) Undamaged. $t = 0.75$ seconds.	(f) Damaged. $t = 0.75$ seconds.
(g) Undamaged. $t = 1.0$ seconds.	(h) Damaged. $t = 1.0$ seconds.

## EXAMPLE: BEAM (2)



## Convergence of Cost Function

#### MOTIVATION: PARAMETER ESTIMATION

- In Many Cases
  - Material Behaviour Unknown
  - Material Parameters Unknown
- Modus Operandi:
  - Load Structure
  - Measure Displacements/Strains
  - For Each Possible Material ModelObtain Best Parameter Set
  - Keep 'Best Fitting' Model

#### ASSUMPTIONS

- Monitoring Via Loads and Measurements:
  - n (Standard) Loads  $\mathbf{f}^i, i = 1, n$  Given
  - n Displacements/Strains Measured at m Locations
- 'Uniform Materials'
- Material Parameters Described by  $\pmb{\beta}$
- Deformations and Strains Well Described by FEM:

$$\mathbf{K} \cdot \mathbf{u}_i = \mathbf{f}_i \quad , \quad i = 1, n \quad , \quad \mathbf{K} = \sum_{el} \mathbf{K}_{el}(\boldsymbol{\beta})$$

## FORMULATE AS OPTIMIZATION PROBLEM

Find  $\boldsymbol{\beta}$  Such That

$$I(\mathbf{u}_{1,\dots,n},\boldsymbol{\beta}) = \frac{1}{2} \sum_{i=1}^{n} \sum_{j=1}^{m} w_{ij}^{md} (\mathbf{u}_{ij}^{md} - \mathbf{I}_{ij}^{d} \cdot \mathbf{u}_{i})^{2}$$
$$+ \frac{1}{2} \sum_{i=1}^{n} \sum_{j=1}^{m} w_{ij}^{ms} (\mathbf{s}_{ij}^{ms} - \mathbf{I}_{ij}^{s} \mathbf{s}_{i})^{2} \to min$$

Subject To:

$$\mathbf{K} \cdot \mathbf{u}_i = \mathbf{f}_i \ , \ i = 1, n \ , \ \mathbf{K} = \sum_{el} \mathbf{K}_{el}(\boldsymbol{\beta})$$

Matparam/Force/Thermo 4

#### EXTEND TO LAGRANGIAN FUNCTIONAL

$$L(\mathbf{u}_{1,..,n},\boldsymbol{\beta},\tilde{\mathbf{u}}_{1,..,n}) = I(\mathbf{u}_{1,..,n},\boldsymbol{\beta}) + \sum_{i=1}^{n} \tilde{\mathbf{u}}_{i}^{t} \cdot (\mathbf{K} \cdot \mathbf{u}_{i} - \mathbf{f}_{i})$$

-  $\tilde{\mathbf{u}}:$  Adjoint Variables

Variation of the Lagrangian WRT Each Component

$$\begin{split} \frac{dL}{d\tilde{\mathbf{u}}_i} &= \mathbf{K} \cdot \mathbf{u}_i - \mathbf{f}_i = 0\\ \frac{dL}{d\mathbf{u}_i} &= \sum_{j=1}^m w_{ij}^{md} \mathbf{I}_{ij}^d \cdot (\mathbf{u}_{ij}^{md} - \mathbf{I}_{ij}^d \cdot \mathbf{u}_i)\\ &+ \sum_{j=1}^m w_{ij}^{ms} \mathbf{J}_{ij}^s \cdot (\mathbf{s}_{ij}^{ms} - \mathbf{I}_{ij}^s \mathbf{s}_i) + \mathbf{K}^t \tilde{\mathbf{u}}_i = 0\\ \frac{dL}{d\boldsymbol{\beta}_i} &= \sum_{i=1}^n \tilde{\mathbf{u}}_i^t \frac{\cdot d\mathbf{K}}{d\boldsymbol{\beta}} \cdot \mathbf{u}_i = \sum_{i=1}^n \tilde{\mathbf{u}}_i^t \frac{\cdot d\mathbf{K}^e}{d\boldsymbol{\beta}} \cdot \mathbf{u}_i \quad, \end{split}$$
-  $\mathbf{J}_{ij}^s$ : Relationship Displacements/Strains

### USE OF ADJOINT SOLVERS

- Consequences
  - Gradient of L wrt  $\boldsymbol{\beta}$ : n Forward/Adjoint Solves;
  - **Independent** of Number of Variables of  $\boldsymbol{\beta}$  (!);
  - Once Forward/Adjoint Solves Done: Cost of Gradient of Each  $\boldsymbol{\beta}$  Is  $O(N_e)$ ;

 $\Rightarrow$  Can Use Detailed FEM Models  $\rightarrow$  Detailed Digital Twin

- 'Hi-Fi Digital Twin'
- Based on Algebraic Equations (FEMs), Not PDEs  $\Rightarrow$  General
- Most Structural Problems:  $\mathbf{K} =^{t} \mathbf{K} \Rightarrow$ 
  - Direct Solvers: Cost of Adjoint Negligible
  - Iterative Solver: Preconditioner Can Be Re-Utilized

#### OPTIMIZATION CYCLE

- For Each Force/Measurement Pair i:
- With Current  $\boldsymbol{\beta}$ : Obtain Displacements  $\rightarrow \mathbf{u}_i$
- With Current  $\boldsymbol{\beta}, \mathbf{u}_i, \mathbf{u}_{ij}^{md}, \mathbf{s}_{ij}^{md}$ : Obtain Adjoints  $\rightarrow \tilde{\mathbf{u}}_i$
- With  $\mathbf{u}_i, \tilde{\mathbf{u}}_i$ : Obtain Gradients  $\rightarrow I^i_{,,\beta} = L^i_{,,\beta}$
- Sum up the gradients  $\rightarrow I_{,\beta} = \sum_{i=1}^{n} I_{,\beta}^{i}$
- If Necessary: Smooth Gradients (Usually Not Needed)  $\rightarrow I^{smoo}_{,\beta}$
- Update  $\boldsymbol{\beta}_{new} = \boldsymbol{\beta}_{old} \gamma I^{smoo}_{,\boldsymbol{\beta}}$ .
- $\gamma$ : Stepsize

#### MOTIVATION: FORCE ESTIMATION

- In Many Cases Forces Unknown
- Modus Operandi:
  - Know Structure/Material Parameters
  - Measure Displacements/Strains
  - Recover Forces

Matparam/Force/Thermo 8

#### ASSUMPTIONS

- Structure Known
  - Materials
  - Material Parameters
- Deformations and Strains Well Described by FEM:

$$\mathbf{K} \cdot \mathbf{u} = \mathbf{f}$$
,  $\mathbf{K} = \sum_{el} \mathbf{K}_{el}$ 

## FORMULATE AS OPTIMIZATION PROBLEM

Find  $\mathbf{f}$  Such That

$$I(\mathbf{u}, \mathbf{f}) = \frac{1}{2} \sum_{j=1}^{m} w_{ij}^{md} (\mathbf{u}_j^{md} - \mathbf{I}_j^d \cdot \mathbf{u})^2$$
$$+ \frac{1}{2} \sum_{j=1}^{m} w_j^{ms} (\mathbf{s}_j^{ms} - \mathbf{I}_j^s \mathbf{s})^2 \to min$$

Subject To:

 $\mathbf{K} \cdot \mathbf{u} = \mathbf{f}$ 

#### EXTEND TO LAGRANGIAN FUNCTIONAL

$$L(\mathbf{u}, \mathbf{f}, \tilde{\mathbf{u}}) = I(\mathbf{u}, \mathbf{f}) + \tilde{\mathbf{u}}^t \cdot (\mathbf{K} \cdot \mathbf{u} - \mathbf{f})$$

-  $\tilde{\mathbf{u}}:$  Adjoint Variables

Variation of the Lagrangian WRT Each Component

$$\begin{aligned} \frac{dL}{d\tilde{\mathbf{u}}} &= \mathbf{K} \cdot \mathbf{u} - \mathbf{f} = 0\\ \frac{dL}{d\mathbf{u}} &= \sum_{j=1}^{m} w_j^{md} \mathbf{I}_j^d (\mathbf{u}_j^{md} - \mathbf{I}_j^d \mathbf{u})\\ &+ \sum_{j=1}^{m} w_j^{ms} \mathbf{J}_j^s (\mathbf{s}_j^{ms} - \mathbf{I}_j^s \mathbf{s}) + \mathbf{K}^t \tilde{\mathbf{u}} = 0\\ \frac{dL}{d\mathbf{f}} &= -\tilde{\mathbf{u}}^t \mathbf{M}_v \end{aligned}$$

### USE OF ADJOINT SOLVERS

- Consequences
  - Gradient of L wrt **f**: One Forward/Adjoint Solve;
  - Independent of Number of Variables of f (!);
  - Once Forward/Adjoint Solves Done: Cost of Gradient of Each **f** Is O(1);

 $\Rightarrow$  Can Use Detailed FEM Models  $\rightarrow$  Detailed Digital Twin

- 'Hi-Fi Digital Twin'
- Based on Algebraic Equations (FEMs), Not PDEs  $\Rightarrow$  General
- Most Structural Problems:  $\mathbf{K} =^{t} \mathbf{K} \Rightarrow$ 
  - Direct Solvers: Cost of Adjoint Negligible
  - Iterative Solver: Preconditioner Can Be Re-Utilized

#### OPTIMIZATION CYCLE

- With Current  $\mathbf{f} \colon \operatorname{Obtain}\,\operatorname{Displacements}\to \mathbf{u}$
- With Current  $\mathbf{f},\,\mathbf{u},\,\mathbf{u}_{j}^{md},\mathbf{s}_{j}^{md}\!\!:$  Obtain Adjoints  $\rightarrow\tilde{\mathbf{u}}$
- With  $\mathbf{u}, \tilde{\mathbf{u}}$ : Obtain Gradients  $\rightarrow I_{,\mathbf{f}} = L_{,\mathbf{f}}$
- If Necessary: Smooth Gradients (Usually Not Needed)  $\rightarrow I^{smoo}_{,{\bf f}}$
- Update  $\mathbf{f}_{new} = \mathbf{f}_{old} \gamma I_{\mathbf{f}}^{smoo}$ .
- $\gamma$ : Stepsize

#### MOTIVATION: TEMPERATURE ESTIMATION

- In Many Cases
  - Temperature Distribution Unknown (Solar Irradiation, Shading, Wind, ...)
  - Material Parameters Known
- Modus Operandi:
  - Load Structure
  - Measure Displacements/Strains
  - Recover Temperature

#### THERMAL EXPANSION/CONTRACTION (1)

- 1-D Rod:
  - Length L
  - Temperature Increment  $\Delta T \Rightarrow$  Expansion  $\Delta L$ :

$$\Delta L = \alpha L \Delta T$$

-  $\alpha$ : Coefficient of Thermal Expansion  $\Rightarrow$  Strain  $\epsilon$ :

$$\epsilon = \alpha \Delta T$$

For Isotropic Thermal Strain:

$$\sigma^{ij} = -\frac{\alpha E}{1-2\nu} \Delta T \delta^{ij}$$

- E: Young's Modulus
- $\nu$ : Poisson Coefficient

## THERMAL EXPANSION/CONTRACTION (2)

Conservation of Momentum (Forces):

$$\sigma^{ij}_{,j} = f$$

Internal Forces  $f_i^k$  Due to Thermal Strain:

$$f_{int}^k = -\left(\frac{\alpha E}{1-2\nu}\Delta T\right)_{,k}$$

## THERMAL EXPANSION/CONTRACTION (3)

FEM/Integration by Parts:

$$\int N^i f_{int}^k d\Omega = \int N^i_{,k} \frac{\alpha E}{1 - 2\nu} N^j \Delta \hat{T}_j d\Omega$$

-  $N^i$ : Shape Function of Node/DOF i

For Linear Elements:

$$\hat{f}_k^i = \sum_{els} N^i_{,k} \frac{\alpha E}{1 - 2\nu} Vol_{el} \Delta T^{ave}_{el} \quad , \quad T^{ave}_{el} = \frac{1}{nn} \sum_{j=1}^{nn} \Delta \hat{T}_j$$

#### ASSUMPTIONS

- Monitoring Via Loads and Measurements:
  - n (Standard) Loads  $\mathbf{f}^i, i = 1, n$  Given
  - n Displacements/Strains Measured at m Locations
- 'Uniform Materials'
- Deformations and Strains Well Described by FEM:

$$\mathbf{K} \cdot \mathbf{u}_i = \mathbf{f}_i \quad , \quad i = 1, n \quad , \quad \mathbf{K} = \sum_{el} \mathbf{K}_{el}$$

## FORMULATE AS OPTIMIZATION PROBLEM

Find  $\Delta \mathbf{T}$  Such That

$$I(\mathbf{u}_{1,\dots,n},\Delta\mathbf{T}) = \frac{1}{2} \sum_{i=1}^{n} \sum_{j=1}^{m} w_{ij}^{md} (\mathbf{u}_{ij}^{md} - \mathbf{I}_{ij}^{d} \cdot \mathbf{u}_{i})^{2}$$
$$+ \frac{1}{2} \sum_{i=1}^{n} \sum_{j=1}^{m} w_{ij}^{ms} (\mathbf{s}_{ij}^{ms} - \mathbf{I}_{ij}^{s} \mathbf{s}_{i})^{2} \to min$$

Subject To:

$$\mathbf{K} \cdot \mathbf{u}_i = \mathbf{f}_i + \mathbf{f}_{\Delta \mathbf{T}} \quad , \quad i = 1, n$$

#### EXTEND TO LAGRANGIAN FUNCTIONAL

$$L(\mathbf{u}_{1,..,n}, \Delta \mathbf{T}, \tilde{\mathbf{u}}_{1,..,n}) = I(\mathbf{u}_{1,..,n}, \Delta \mathbf{T})$$
$$+ \sum_{i=1}^{n} \tilde{\mathbf{u}}_{i}^{t} \cdot (\mathbf{K} \cdot \mathbf{u}_{i} - \mathbf{f}_{i} - \mathbf{f}_{\Delta \mathbf{T}})$$

-  $\tilde{\mathbf{u}}:$  Adjoint Variables

Variation of the Lagrangian WRT Each Component

$$\frac{dL}{d\tilde{\mathbf{u}}_{i}} = \mathbf{K} \cdot \mathbf{u}_{i} - \mathbf{f}_{i} - \mathbf{f}_{\Delta \mathbf{T}} = 0$$
$$\frac{dL}{d\mathbf{u}_{i}} = \sum_{j=1}^{m} w_{ij}^{md} \mathbf{I}_{ij}^{d} \cdot (\mathbf{u}_{ij}^{md} - \mathbf{I}_{ij}^{d} \cdot \mathbf{u}_{i})$$
$$+ \sum_{j=1}^{m} w_{ij}^{ms} \mathbf{J}_{ij}^{s} (\mathbf{s}_{ij}^{ms} - \mathbf{I}_{ij}^{s} \mathbf{s}_{i}) + \mathbf{K}^{t} \tilde{\mathbf{u}}_{i} = 0$$
$$\frac{dL}{d\Delta \mathbf{T}} = \sum_{i=1}^{n} \tilde{\mathbf{u}}_{i}^{t} \frac{d\mathbf{f}_{\Delta \mathbf{T}}}{d\Delta \mathbf{T}}$$

-  $\mathbf{J}_{ij}^s:$  Relationship Displacements/Strains

### USE OF ADJOINT SOLVERS

- Consequences
  - Gradient of L wrt  $\Delta \mathbf{T}$ : n Forward/Adjoint Solves;
  - Independent of Number of Variables of  $\Delta T$  (!);
  - Once Forward/Adjoint Solves Done: Cost of Gradient of Each  $\Delta T_i$  Is O(1);

 $\Rightarrow$  Can Use Detailed FEM Models  $\rightarrow$  Detailed Digital Twin

- 'Hi-Fi Digital Twin'
- Based on Algebraic Equations (FEMs), Not PDEs  $\Rightarrow$  General
- Most Structural Problems:  $\mathbf{K}=^t\mathbf{K}\Rightarrow$ 
  - Direct Solvers: Cost of Adjoint Negligible
  - Iterative Solver: Preconditioner Can Be Re-Utilized

#### OPTIMIZATION CYCLE

- For Each Force/Measurement Pair i:
- With Current  $\Delta \mathbf{T}$ : Obtain Displacements  $\rightarrow \mathbf{u}_i$
- With Current  $\Delta \mathbf{T}, \mathbf{u}_i, \mathbf{u}_{ij}^{md}, \mathbf{s}_{ij}^{md}$ : Obtain Adjoints  $\rightarrow \tilde{\mathbf{u}}_i$
- With  $\mathbf{u}_i, \tilde{\mathbf{u}}_i$ : Obtain Gradients  $\rightarrow I^i_{,\Delta \mathbf{T}} = L^i_{,\Delta \mathbf{T}}$
- Sum up the gradients  $\rightarrow I_{,\Delta \mathbf{T}} = \sum_{i=1}^{n} I_{,\Delta \mathbf{T}}^{i}$
- If Necessary: Smooth Gradients (Usually Not Needed)  $\rightarrow I^{smoo}_{,\Delta \mathbf{T}}$
- Update  $\Delta \mathbf{T} new = \Delta \mathbf{T} old \gamma I_{,\Delta \mathbf{T}}^{smoo}$ .
- $\gamma$ : Stepsize

# **Plate With Hole (MP1)**

- Linear Elastic
- Material Parameters: E, v
- 4, 6 and 14 Sensors







# **Plate With Hole (MP2)**



## Plate With Hole (MP3a)

4 Sensors



# **Plate With Hole (MP3b)**

6 Sensors



# **Plate With Hole (MP3c)**

14 Sensors


## Hoover Dam (MP1)

- Linear Elastic
- Material Parameters: E, v
- Mesh and Target Displacement



### Hoover Dam (MP2)



### Hoover Dam (MP3)

• Convergence History



### **Truss (MP1)**

- Nonlinear Material
  - $\epsilon \le \epsilon_0 : E = E_0$

 $\epsilon \ge \epsilon_0 : E_0 \epsilon_0 + (E_1 \epsilon_1 - E_0 \epsilon_0) (\log(\epsilon/\epsilon_0)) / (\log(\epsilon_1/\epsilon_0))$ 



### Truss (MP2)

- 4 Load Cases:  $F_x = (10^5, 4 \cdot 10^7, 5 \cdot 10^7, 6 \cdot 10^7)$
- Target:
- $E_0=2{\cdot}\,10^{11}$  ,  $E_1=1.6{\cdot}\,10^{10}$
- $\varepsilon_0 = 2 \cdot 10^{-3}$ ,  $\varepsilon_1 = 5 \cdot 10^{-2}$
- v = 0.3
- 4 Sensors Along the Truss



### **Truss (MP3)**

• Convergence History



# **Bridge: 8 Sensors**



### FEELAST



# **Bridge: 36 Sensors**



### Material: Steel Trusses: A=1-100cm<sup>2</sup> FEELAST



#### USE OF REDUCED BASIS (1)

- Detailed FEM Model Not Available/Possible

 $\Rightarrow$ 

- Use Reduced Basis Models

Options:

- Limited (Small) Number of Eigenmodes
- Proper Orthogonal Decompositions (POCs)
- Reduced Order Methods (ROMs)

#### EIGENMODES (1)

Original FEM/System:

 $\Rightarrow$ 

#### $\mathbf{K}\cdot\mathbf{u}=\mathbf{f}$

System of Eigenvalues  $\lambda$  and Eigenvectors **e**:

$$\mathbf{K} \cdot \mathbf{e} = \lambda \mathbf{e}$$

 $\mathbf{K}\cdot\mathbf{E}=\mathbf{E}\cdot\boldsymbol{\Lambda}$ 

-  $\mathbf{E}$ : Rows of Eigenvectors

-  $\Lambda$ : Diagonal Matrix of Eigenvalues

#### EIGENMODES (2)

Multiplication From Right by  $\mathbf{E}^{-1} = \mathbf{E}^t$ (**K** Symmetric and Positive Definite):

 $\mathbf{K} = \mathbf{E} \cdot \mathbf{\Lambda} \cdot \mathbf{E}^t$ 

Changing Variables and Forces via Projection:

$$\mathbf{u} = \mathbf{E} \cdot \mathbf{v}$$
 ,  $\mathbf{f} = \mathbf{E} \cdot \mathbf{f}^e$ 

 $\Rightarrow$ 

#### $\mathbf{\Lambda} \cdot \mathbf{v} = \mathbf{f}^e$

 $\Rightarrow$  **Uncoupled** System of Equations for Eigenmodes Similar Decompositions Used for PODs and ROMs.

### WEAKNESS DETECTION (1)

#### Lagrangian Functional Written With New Variables

$$L = \frac{1}{2} \sum_{i=1,m} w_i^{md} \left( \mathbf{u}^m(\mathbf{x}_i) - \mathbf{u}^{FEM}(\mathbf{x}_i) \right)^2 + \tilde{\mathbf{u}}^t \cdot (\mathbf{K} \cdot \mathbf{u} - \mathbf{f})$$

$$L = \frac{1}{2} \sum_{i=1}^{m} w_i^{md} \left( \mathbf{u}^{md}(\mathbf{x}_i) - \mathbf{E}(\mathbf{x}_i) \cdot \mathbf{v} \right)^2 + \tilde{\mathbf{v}}^t \cdot \left( \mathbf{\Lambda} \cdot \mathbf{v} - \mathbf{f}^e \right)$$

Derivatives:

$$\frac{dL}{d\tilde{\mathbf{v}}} = \mathbf{\Lambda} \cdot \mathbf{v} - \mathbf{f}^e = 0$$

$$\frac{dL}{d\mathbf{v}} = \sum_{i=1}^{m} w_i^{md} \mathbf{E}(\mathbf{x}_i) (\mathbf{u}^{md}(\mathbf{x}_i) - \mathbf{E}(\mathbf{x}_i) \cdot \mathbf{v}) + \mathbf{\Lambda} \cdot \tilde{\mathbf{v}} = 0$$

#### WEAKNESS DETECTION (2)

$$\frac{dL}{d\alpha_e} = \tilde{\mathbf{v}}^t \cdot \frac{d\mathbf{\Lambda}}{d\alpha_e} \cdot \mathbf{v}$$

Derivative of Each Eigenvalue  $\lambda_i$  WRT  $\alpha_e$  (Appendix 1):

$$\frac{d\lambda_i}{d\alpha_e} = \frac{\mathbf{e}_i^t \cdot \mathbf{K}_e \cdot \mathbf{e}_i}{\mathbf{e}_i^t \cdot \mathbf{e}_i}$$

 $\Rightarrow$  Need Stiffness Matrix of Each Element BUT: May Not Be Available !

For Forces:

$$\frac{dL}{d\mathbf{f}_i^e} = -\tilde{\mathbf{v}}^t \cdot \frac{d\mathbf{f}^e}{d\mathbf{f}_i^e} = -\tilde{\mathbf{v}}_i^t$$

Use of Reduced Basis 6

#### APPENDIX 1: DERIVATIVES OF EIGENVALUES (1)

Original System:

$$\mathbf{K} \cdot \mathbf{e} = \lambda \mathbf{e} \tag{(*)}$$

Change In Stiffness:

$$\Delta \mathbf{K}_e = \Delta \alpha_e \mathbf{K}_e$$

 $(\mathbf{K} + \Delta \mathbf{K}) \cdot (\mathbf{e} + \Delta \mathbf{e}) = (\lambda + \Delta \lambda)(\mathbf{e} + \Delta \mathbf{e})$ 

Expand:

 $\Rightarrow$ 

$$\mathbf{K} \cdot \mathbf{e} + \Delta \mathbf{K} \cdot \mathbf{e} + \mathbf{K} \cdot \Delta \mathbf{e} + \Delta \mathbf{K} \cdot \Delta \mathbf{e} = \lambda \mathbf{e} + \lambda \Delta \mathbf{e} + \Delta \lambda \mathbf{e} + \Delta \lambda \Delta \mathbf{e}$$

Use of Reduced Basis 7

### APPENDIX 1: DERIVATIVES OF EIGENVALUES (2)

Removing 0th Order Terms, Neglect 2nd Order Terms:

$$\Delta \mathbf{K} \cdot \mathbf{e} + \mathbf{K} \cdot \Delta \mathbf{e} = \lambda \ \Delta \mathbf{e} + \Delta \lambda \ \mathbf{e} \tag{**}$$

Multiply from Left by  $\mathbf{e}^t \Rightarrow$ 

$$\mathbf{e}^{t} \cdot \Delta \mathbf{K} \cdot \mathbf{e} + \mathbf{e}^{t} \cdot \mathbf{K} \cdot \Delta \mathbf{e} = \mathbf{e}^{t} \cdot \lambda \cdot \Delta \mathbf{e} + \mathbf{e}^{t} \cdot \Delta \lambda \cdot \mathbf{e}$$

Use Eqn.(\*) Again:

$$\mathbf{e}^t \cdot \Delta \mathbf{K} \cdot \mathbf{e} = \mathbf{e}^t \cdot \Delta \lambda \cdot \mathbf{e}$$

 $\Rightarrow$ 

$$\mathbf{e}^t \cdot \Delta \alpha_e \mathbf{K}_e \cdot \mathbf{e} = \mathbf{e}^t \cdot \Delta \lambda \cdot \mathbf{e}$$

$$\frac{\Delta\lambda}{\Delta\alpha_e} = \frac{\mathbf{e}^t \cdot \mathbf{K}_e \cdot \mathbf{e}}{\mathbf{e}^t \cdot \mathbf{e}}$$

### APPENDIX 1: DERIVATIVES OF EIGENVALUES (3)

And  $\frac{\Delta \mathbf{e}}{\Delta \alpha_e}$  ? Using Eqn.(\*\*):

$$(\mathbf{K} - \lambda \mathbf{I}) \cdot \Delta \mathbf{e} = (\Delta \lambda - \Delta \mathbf{K}) \cdot \mathbf{e}$$

 $\Rightarrow$ 

$$(\mathbf{K} - \lambda \mathbf{I}) \cdot \frac{\Delta \mathbf{e}}{\Delta \alpha_e} = \left(\frac{\Delta \lambda}{\Delta \alpha_e} - \mathbf{K}_e\right) \cdot \mathbf{e}$$

Seems Costly

If Decomposition of  ${\bf K}$  Given: Iterative Scheme

$$\mathbf{K} \cdot \left(\frac{\Delta \mathbf{e}}{\Delta \alpha_e}\right)^{i+1} = \lambda \mathbf{I} \cdot \left(\frac{\Delta \mathbf{e}}{\Delta \alpha_e}\right)^i + \left(\frac{\Delta \lambda}{\Delta \alpha_e} - \mathbf{K}_e\right) \cdot \mathbf{e}$$

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